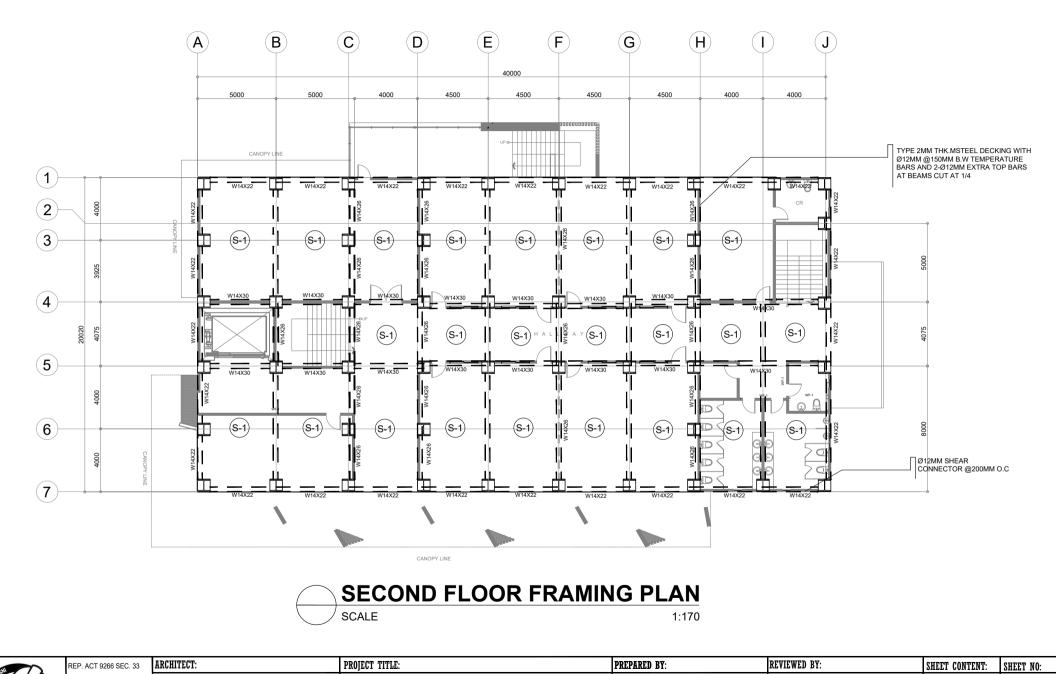
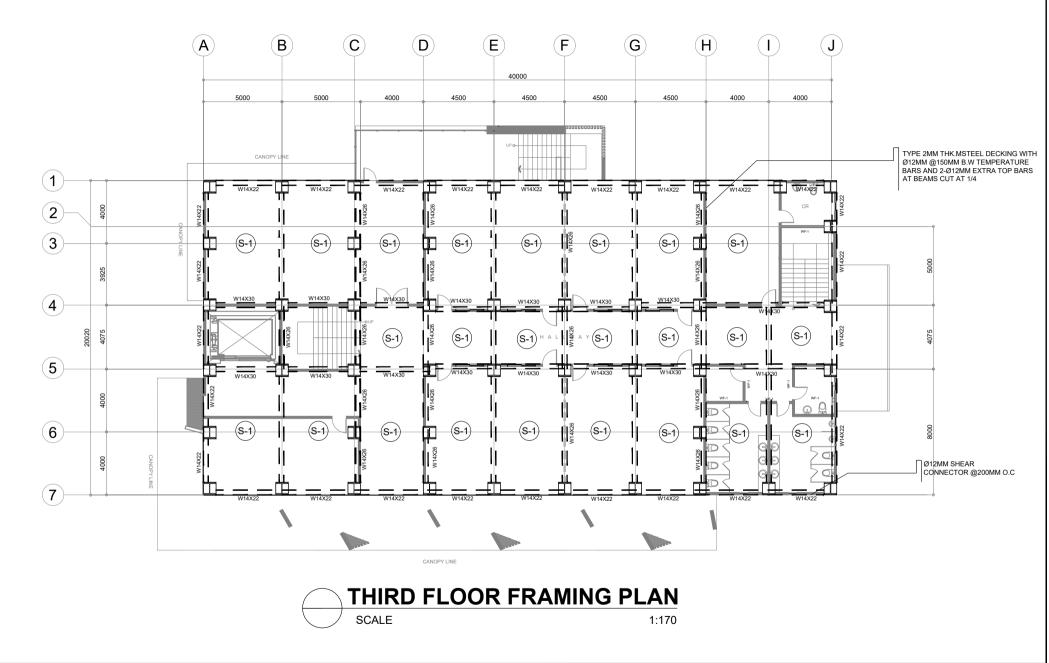


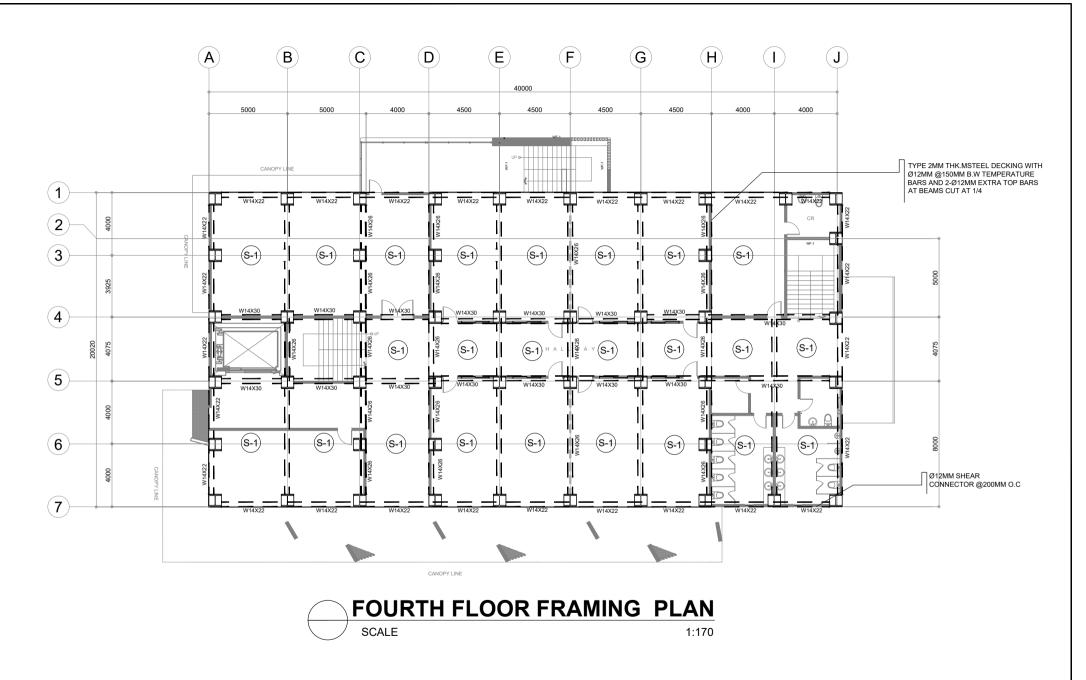
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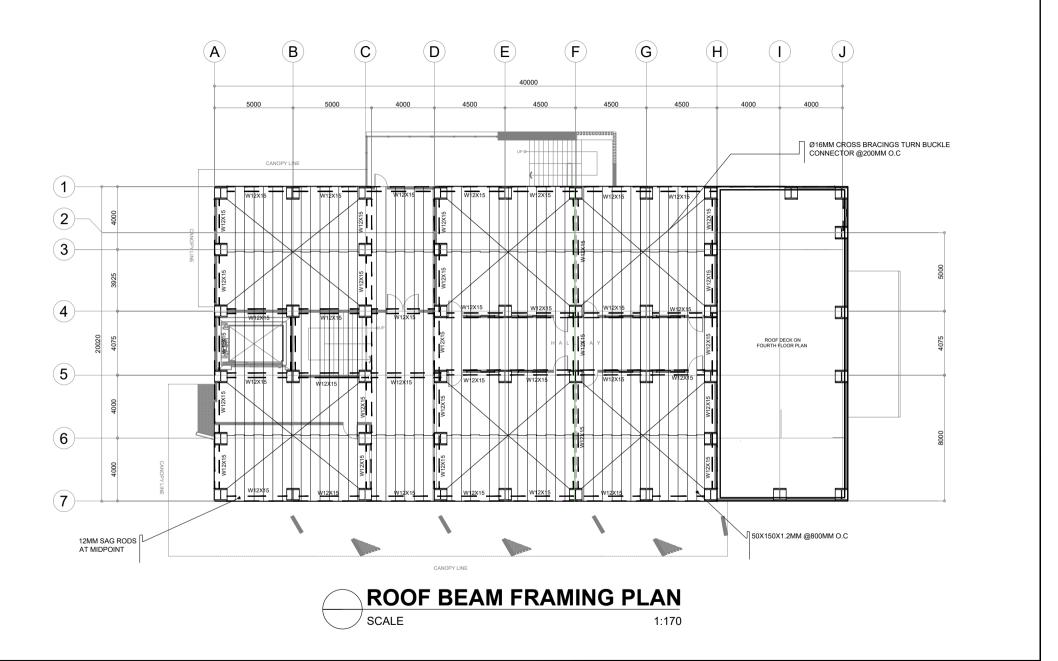
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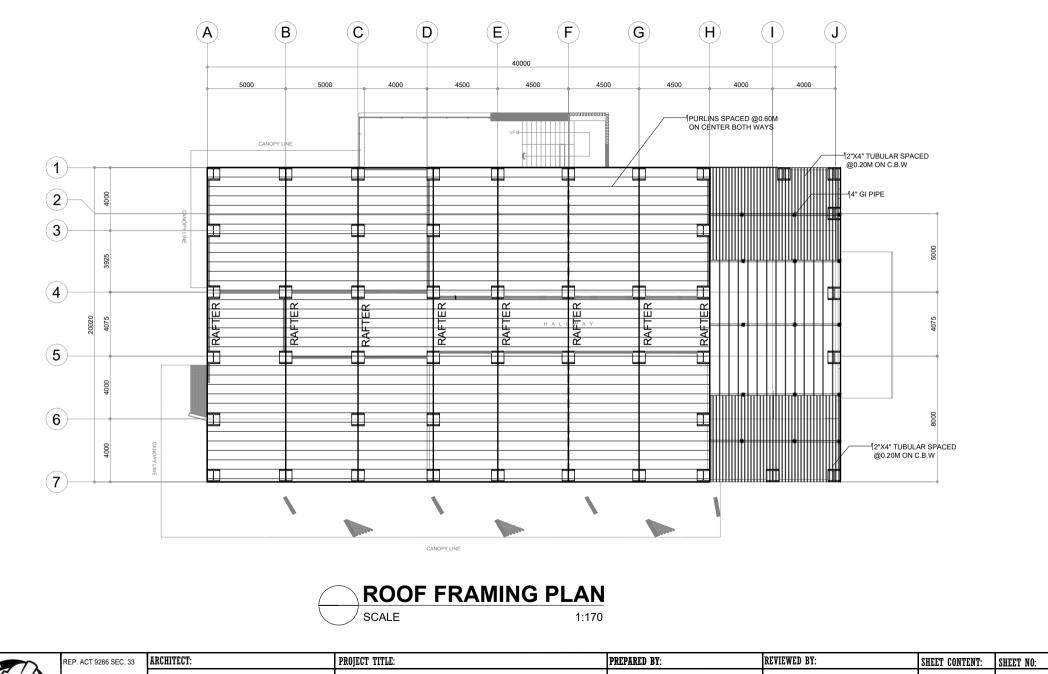
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		Place :	TIN :	LOCATION: ROMBLON STATE UNIVERSITY-MAIN CAMPUS RE	Unit Head, PPDU-IPPDO	Romblon State University President			1

A. DESIGN CRITERIA

1. DESIGN REFERENCES

- A. AMERICAN CONCRETE INSTITUTE (ACI PUBLICATIONS); ACI 318-08M BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRET.
- ACI DETAILING MANUAL 2004 FOR DETAILS AND DETAILING OF CONCRETE REINFORCEMENT B. AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) PUBLICATION. MANUAL OF STEEL CONSTRUCTION, THIRTEENTH EDITION. "AISC 360-05"
- C. AMERICAN WELDING SOCIETY (AWS) PUBLICATION D.1.1-2000. D. AMERICAN SOCIETY FOR TESTING MATERIALS (ASTM)
- E. NATIONAL STRUCTURAL CODE OF THE PHILIPPINES (NSCP 2015) VOL. 1, 6TH EDITION
- F. ASSOCIATION OF STRUCTURAL ENGINEERS OF THE PHILIPPINES (ASEP) HANDBOOK OF STRUCTURAL STEEL SHAPES AND SECTIONS, 1994
- G. UNIFORM BUILDING CODE (UBC), VOL. 2 1997 EDITION

DESIGN LOADS A DEAD LOADS (DL) :

<i>n</i> .	DL	RD LOADS (DL) .		
	A.1	CONCRETE		kN
	A.2	STRUCTURAL STEEL ·····		kN
	A.3	SOIL	18.00	kP
	A.4	CEILING		kP
	A.5	FLOOR FINISH		kP
	A.6	MOVABLE PARTITION		kP
	A.7	MECH/ELEC DUCT	0.20	kP
	A.8	PARITITION	0.50	kP
в.	υv	E LOADS (LL) :		
	B.1	WARDS AND ROOMS	1.90	kР
	B.2	LAB. AND OPERATING ROOMS · · · · · · · · · · · · · · · · · · ·	2.90	kP
	B.3	CORRIDORS ABOVE GROUND FLOOR	3.80	kP
	B.4	OFFICE		kP
	B.5	COMFORT ROOM ·····		kР
	B.6	LIGHT STORAFE		kP
	B.8	ROOF	1.00	νD

C. LATERAL LOADS

C.1 WIND LOAD (WL) THE WIND LOAD ON STRUCTURE AND BUILDING SHALL BE CALCULATED, BASED ON NATIONAL STRUCTURAL CODE OF THE PHILIPPINES, CONSIDERING BASIC WIND SPEED

ASIC WIND SPEED, V = 270 KM/hour OCCUPANCY CATEGORY = 1 UMPORTANCE FACTOR, w = 1.0 EXPOSURE CATEGORY = 4.3x10-6.Kz.Kzt.Vz.lw

THIS VELOCITY PRESSURE SHALL BE USED ALONG WITH FORCE COEFFICIENTS TO CALCULATE WIND LOAD ON SPECIFIC STRUCTURE.

C.2 SEISMIC LOAD , E SEISMIC LOADS FOR BUILDING STRUCTURES ARE CALCULATED BASED ON THE FOLLOWING:

 $E = {}_{\Gamma}Eh + Ev$ Em = VoEh

- WHERE-
- E = EARTHQUAKE LOAD ON THE STRUCTURE
- $\mathsf{Eh}=\mathsf{THE}\ \mathsf{EARTHQUAKE}\ \mathsf{LOAD}\ \mathsf{DUE}\ \mathsf{TO}\ \mathsf{THE}\ \mathsf{BASE}\ \mathsf{SHEAR},\ \mathsf{V},\ \mathsf{OR}\ \mathsf{THE}\ \mathsf{DESIGN}\ \mathsf{LATERAL}\ \mathsf{FORCE}\ \mathsf{Fp}.$

 $\mathsf{Em}=\mathsf{THE}\;\mathsf{ESTIMATED}\;\mathsf{MAXIMUM}\;\mathsf{EARTHQUAKE}\;\mathsf{FORCE}\;\mathsf{THAT}\;\mathsf{CAN}\;\mathsf{BE}\;\mathsf{DEVELOPED}\;\mathsf{IN}\;\mathsf{THE}\;\mathsf{STRUCTURE}.$

DEVELOPED IN THE SINUCIONE. EVEN THE LOAD EFFECT RESULTING FROM THE VERTICAL COMPONENT OF THE EARTHQUAKE GROUND MOTION AND IS EQUAL TO AN ADDITIONAL OF 0.5 CG I D TO THE DEAD LOAD EFFECT. D, FOR STRENGTH DESIGN, AND MAY BE TAKEN AS ZERO FOR ALLOWABLE STRESS DESIGN.

Vo = THE SEISMIC FORCE AMPLIFICATION FACTOR THAT IS REQUIRED TO ACCOUNT FOR STRUCTURAL OVERSTRENGTH.

r = RELIABILITY/REDUNDANCY FACTOR WHICH SHALL NOT BE TAKEN LESS THAN 1.0 AND GREATER THAN 1.5, IS GIVEN BY THE FOLLOWING FORMULA: 61

WHERE:

 $r = 2 \frac{0.1}{\text{rmax}\sqrt{AB}}$ THICK-THE MAXIMUM ELEMENT-STORY SHEAR RATIO. FOR A GIVEN DIRECTION OF LOADING, THE ELEMENT-STORY SHEAR RATIO IS THE RATIO OF THE DESIGN OF STORY SHEAR IN THE HEAVILY LOADED SINGLE ELEMENT DIVIDED BY THE TOTAL DESIGN STORY SHEAR.

FOR MOMENT FRAMES, IT SHALL BE TAKEN AS THE MAXMUM OF THE SUM OF THE SHEARS IN ANY TWO ADJACENT COLUMNS IN A MOMENT FRAME BAY DIVIDED BY THE STORY SHEAR. FOR COLUMNS COMMON TO TWO BAYS, 70 FERCENT, OF THE SHEAR IN THAT COLUMN MAY BE USED IN THE

AR = THE GROUND FLOOR AREA OF THE STRUCTURE.

EARTHQUAKE BASE SHEAR, (V) THE TOTAL DESIGN BASE SHEAR IN A GIVEN DIRECTION SHALL BE DETERMINED FROM THE FOLLOWING FORMULA: V = Cv | (W)

AND NEED NOT EXCEED THE FOLLOWING:

 $V = \frac{2.5 \text{ Cal}}{\text{R}} (\text{W})$

BUT SHALL NOT BE LESS THAN THE FOLLOWING: V = 0.11 Ca | W

IN ADDITION FOR SEISMIC ZONE 4, THE TOTAL BASE SHALL NOT LESS ALSO BE THAN THE FOLLOWING:

= 1.072 = 1.00

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V = 8.5 Z Nv + (W)WHERE:

SEISMIC ZONE FACTOR, Z = 0.40 IMPORTANCE FACTOR 1 = 1.5 GLOBAL DUCTILITY CAPACITY R = 8.0 (SMRF) SEISMIC FORCE OVERSTRENGTH FACTOR Vo = 2.8 SEISMIC SOURCE TYPE = A NEAR - SOURCE FACTOR, NV NEAR - SOURCE TYPE, Na

- B. GENERAL NOTES:
- A. IN THE INTERPRETATION OF THESE DRAWINGS, INDICATED DIMENSIONS SHALL GOVERN AND DISTANCES OR SIZES SHALL NOT BE SCALED FOR CONSTRUCTION PURPOSES.
- B. THE CONTRACTOR SHALL COORDINATE WITH THE AR, ME, SE, EE, AND OTHER UTILITIES AND EQUIPMENT PLANS FOR THE EXACT SIZE, NUMBER AND LOCATIONS OF ALL EVES OR OPENINGS THRU FLOOR SLABS, SLE BEAMS AND WALLS AND ALSO BUILDING DIMENSIONS.

C. BUT LIMITED TO AND SHALL NOT VIOLATE ON THE REQUIREMENT OF THESE PLAN, SEE NOTES ON PIPE SLEEVES. D. ALL REINFORCED CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH THE ACI-318-05 BUILDING CODE, AND ALL STRUCTURAL STEEL WORK SHALL BE DONE IN ACCORDANCE WITH AISC SPECIFICATIONS

(LATEST EDITION) IN SO FAR AS THEY DO NOT CONFLICT WITH THE LOCAL BUILDING CODE REQUIREMENTS.

(LATES) EDITION) IN SU TAY AS THET DU NUT CARPELT WITH THE EUCLAS BUILDING CODE RECOMMENDENTS. E. ALL SLASE, BEAMS, GRORERS NO D'HET STUCTURAL ELEMENTS WICH ARE NOT INDICATED, DETAILED, DESIGNATED OR INADRETENTLY OMITED BUT ARE NECESSARY TO BE CORDINATED WITH ARCHITECTURAL AND OTHER ALLED ENGINEERING PLANS AS WILL AS TO COMPLET THE STRUCTURAL WORKS IN ACCORDANO WITH THE INTENT OF THE PLANS AND SPECIFICATIONS SHALL BE BROUGHT UP DURING PRE-BIOS/MEDITIOS/ NECOTIATIONS. IT IS UNCERSION THAT THE COMPLETANCIDIRE AND NOLED ALL THESE ITEMS N THEIR BIL

- C. NOTES ON CONCRETE MIXES AND PLACING
- A. UNLESS OTHERWISE INDICATED IN PLANS OR NOTED IN THE SPECIFICATIONS THE MINIMUM 28-DAYS CYUNDER COMPRESSIVE STRENGTH OF CONCRETE C, SHALL BE AS FOLLOWS:

۸.	FOUNDATION, STP,	CISTERN TANK	fc' = 3000 psi
В.	COLUMNS, BEAMS,	GIRDERS AND SUSPENDED SLAB, RC WALL	fc' = 3000 psi
с.	SLAB ON GRADE		fc' = 3000 psi
D.	RETAINING WALL		fc' = 3000 psi

B. CONGRETE SHALL BE DEPORTED IN THE FAML POSTDIM WITHOUT SEGREGATION, RE-HARDING OF FLAMMA, CAUCINE OF AND A CONSTRUCTION OF THE AND A CONSTRUCT SEGREGATION, RE-HARDING OF THE ALLORED EXCEPT TO TRANSFER CONCRETE FROM HOPPEDS TO BURGES, MUEEL BARROWS OF BURGES, IN MINO-CASE, THEY SHALL NOT EXCEPTION THAN AND A CONSTRUCTION OF THE CONCRETE SHALL NOT EXCEPTION THE USE OF WIRKARDS UNLESS AUTHORIZED IN WIRTING BY THE CONCRETE SHALL NOT EXCEPTION THE USE OF WIRKARDS WILLESS AUTHORIZED IN WIRTING BY THE CONCRETE SHALL BE ALLOWED WITHOUT THE USE OF WIRKARDS WILLESS AUTHORIZED IN WIRTING BY THE CONCRETE SHALL BE ALLOWED WITHOUT THE USE OF WIRKARDS WILLESS AUTHORIZED IN WIRTING BY THE CONCRETE SHALL BE ALLOWED WITHOUT THE USE OF WIRKARDS WILLESS AUTHORIZED IN WIRTING BY THE CONCRETE SHALL BE AUTOMIC ON THE USE OF WIRKARDS WIREARDS TO EXCEMPTION.

D. NOTES ON REINFORCING STEEL BARS

A. ALL REINFORCING STEEL BARS SHALL BE NEW BILLET, HOT ROLLED, WELDABLE, DEFORMED BARS CONFORMING TO THE SPECIFICATIONS OF PNS 49: 1986 (ASTM 615) WHOSE GRADE IS SHOWN ON TABLE 1.

TABLE-1							
GRADE	BAR DIAMETER						
GRADE 275 (Fy = 40 ksi)	12MMØ & BELOW						
GRADE 275 (Fy = 40 ksi)	16MMØ & ABOVE						

B. THE SUPPLEMENTARY REQUIREMENTS OF WELDABLE DEFORMED REINFORCING BARS SHALL BE AS FOLLOWS:

1. THE MAXIMUM YIELD STRENGTH OF WELDABLE BARS = 540 MPa.

- THE TENSILE STRENGTH SHALL NOT BE LESS THAN 1.25 TIMES THE ACTUAL YIELD STRENGTH.
- C. ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS, SECURED IN THE RECURRED LOCATION IN ACCORPANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE BUILDING CODE AND THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCERCE STRUCTURES, AG 335.
- D. ALL REINFORCING BARS SHALL BE CLEANED THOROUGHLY OF ALL LOOSE RUST, SOIL OR OTHER MATERIAL IMMEDIATELY PRIOR TO PLACING CONCRETE.
- E. THE REQUIRED LENGTH OF LAP FOR TENSION SPLICES IS BASED ON THE DEVELOPMENT LENGTH, LD, SHOWN IN TABLE 2 AND TABLE 3 FOR RC BEAMS AND GRDERS, RESPECTIVELY AND ON THE FOLLOWING CLASSIFICATIONS: SPLICE LENGTH

TENSION SPLICE CLASSIFICATION CLASS A 1.0 Ld CLASS B 1314

		TABLE-2										
		DEVELOPMENT LENGTH, Ld, in TENSION										
AR SIZE	f'c=20 (3,00		f'c=27.6 MPa (4.000psi)		f c=34.5 MPa (5,000psi)		f'c=41.4 MPa (6,000psi)		f'c=55.2 MPa 8,000psi)		f'c=69.0 MPc (10,000psi)	
(mm)	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHEF BARS
10ø	435	220	375	200	340	200	310	200	300	200	300	200
12ø	520	265	450	230	405	215	370	215	320	215	300	215
16ø	695	350	600	305	540	285	490	285	425	285	380	285
20ø	1085	440	755	380	840	360	615	360	665	360	475	360
25ø	1675	685	1450	595	1360	560	1185	560	1025	560	920	560
28ø	1875	765	1625	665	1455	625	1325	625	1150	625	1030	625
32ø	2145	875	1855	760	1660	715	1575	715	1315	715	1175	715

36ø 2410 965 2090 855 1870 805 1705 805 1475 805 1320 805 F. TOP BARS ARE HORIZONTAL BARS WITH AT LEAST 300 mm OF CONCRETE CAST BELOW IT.

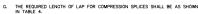
	TABLE-3											
		S	PLICE	LENG	TH, L	st, in	TENS	ION				
BAR SIZE		.7 MPa IOpsi)		.6 MPa Opsi)	f c=34.5 MPa (5.000psi)			.4 MPa IOpsi)	f'c=55.2 MPa 8.000psi)		f'c=69.0 MPa (10,000psi)	
(mm)	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHE BAR
10ø	390	300	490	300	440	300	400	300	390	300	390	300
12ø	680	360	585	360	525	360	480	360	415	360	390	360
16ø	905	480	785	480	700	480	640	480	555	480	495	480
20ø	1130	600	980	600	1095	600	800	600	865	600	620	600
25ø	1740	750	1885	1085	1685	1085	1540	1085	1335	1085	1195	108
28ø	1950	835	2110	1215	1890	1215	1725	1215	1495	1215	1335	1215
32ø	2230	955	2410	1385	2160	1385	1970	1385	1205	1385	1525	138
36ø	2510	1075	2715	1560	2425	1560	2215	1560	1920	1560	1715	156

PROJECT TITLE:

CONSTRUCTION OF COLLEGE OF BUSINESS, ACCOUNTANCY AND

TECHNOPRENEURSHIP BUILDING

LOCATION: ROMBLON STATE UNIVERSITY-MAIN CAMPUS



	TABL	E-4							
LENGTH OF LAP COMPRESSION SPLICES(mm)									
BAR SIZE (mm)	f'c = 28.0 MPa (4,000psi)	f'c = 34.5 MPa (5,000psi)	f'c = 41.4 MPq (6,000psi)						
16	320	320	320						
20	600	600	600						
25	750	750	750						
28	820	820	820						
32	950	950	950						
35	1050	1050	1050						

A FULL WELDED SPLICES SHALL HAVE BARS BUTTED AND WELDED TO DEVELOP IN TENSION AT LEAST 125 PERCENT OF THE SPECIFIED YIELD STRENGTH 1/y, OF THE BAR. (SEE FIGURE 1)

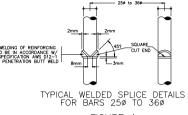


FIGURE 1

ALL WELDING OF REINFORCEMENT SHALL CONFORM TO THE PROVISIONS OF THE STRUCTURAL WELDINGCODE-REINFORCING STEEL, ANS D1.4

- A FULL MECHANICAL CONNECTION (REBAR SPLICER) SHALL DEVELOP IN TENSION OR COMPRESSION, AS REQUIRED, 125 PERCENT OF THE SPECIFIED YELD STENGTH FY, OF THE BAR. IF USED, SUBMIT SAMPLE FOR APPROVAL OF THE STRUCTURAL ENGINEER.
- K. CLEAR CONCRETE COVER FOR REINFORCING BARS SHALL BE AS FOLLOWS:

1)	CONCRETE CAST AGAINST EARTH	– 75 mm
2)	CONCRETE EXPOSED TO EARTH OR WEATHER-	
	20 mm TO 36 mm BARS	– 50 mm
	16 mm BARS AND SMALLER	– 40 mm
T)	CONCRETE NOT EXPOSED TO EARTH OR WEATHER-	

SLABS, WALLS, JOINTS - 20 mm BEAMS AND COLUMNS - 40 mm

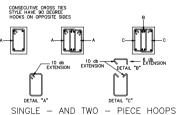


FIGURE 2 I REBAR HOOKS

 A) HOOKS SHALL BE USED IN REBAR AT
i) ALL THE ENDS OF SIMPLE BEAMS.
ii) ALL THE ENDS OF TENSION REBAR AND CANTILEVER BEAMS. REBAR HOOKS MUST BE CONSTRUCTED USING COLD METHODS.

C) HOOK SHAPES MUST BE ADJUSTED TO SUIT TABLE 5.

REV.:

F	HOOK SHAPES AT THE END OF REBAR.						
ANGLE	SHAPE	STEEL Ø (mm)	Ø OF INSIDE HOOK (d)	REBAR END LENGTH (A)			
180 DEGREES	₩	≤25	6d	4d OR 65 MINIMUM			
		>25 AND ≤32	8d				
135 DEGREES		≤16	4d	6d OR 75 MINIMUM			
(STIRRUPS & TIES)	,⊒ E e)″	>16 AND ≤25	6d				
	_, X11√	≤25	6d	12d			
90 DEGREES	୲୕ୣୣଢ଼ୖୖ	>25 AND ≤32	8d				
NSTALLATION AND SU	PERVISION						

PREPARED BY:

CHECKED BY:

FAYE MICHELLE A. BANIAGO

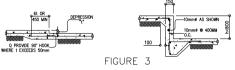
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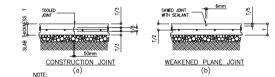
ENGR. NOEL M. TIANGA JR.

EVERY REBAR INTERSECTION IS TO BE TIED WITH STEEL WIRE. TO RETAIN THE DISTANCE BETWEEN REBARS, USE SPACER REBARS.

- C. MIN. DISTANCE BETWEEN REBARS IS THE GREATER OF: 1.5 x d OR - FOR COMPRESSION MEMBERS
- 40mm 1 x d 0R FOR OTHERS 25mm
- D. MAXIMUM DISTANCE BETWEEN REBAR IN WALLS AND SLAB: 3 X MEMBER THICKNESS OR 450mm FOR BUNDLED BARS, perfer to NSCR. 2001
- E. NOTES ON SLAB-ON-GRADE
 - A. THE SOIL SUBGRADE AND FILL LAYERS BELOW ALL SLABS ON GRADE, PAVING AND PIT SLABS SHALL BE MECHANICALLY COMPACTED IN LAYERS TO A MINIMUM OF 95 PERCENT OF THE MODIFIED PROCTOR DENSITY, PER ASTM D1557.

 - B. ALL SLABS-ON-GRADE SHALL BE PROVIDED WITH A MINIMUM OF 100MM THK. COMPACTED CLEAN COARSE GRAVEL BED.
 - 2.1 THE MAXIMUM YIELD STRENGTH OF WELDABLE BARS = 540 MPg. 2.2 THE TENSILE STRENGTH SHALL NOT BE LESS THAN 1.25 TIMES THE ACTUAL YIELD STRENGTH.
 - C. UNLESS OTHERWISE NOTED, ALL BEDDED SLABS SHALL BE REINFORCED WITH 10MM BARS AT 250MM O.C. EACHWAY AT THE CENTER OF SLAB.
 - D. PLACE CONCRETE FOR ALL SLABS-ON-GRADE IN CHECKERBOARD FASHION BETWEEN CONSTRUCTION JOINTS IN AREAS NOT TO EXCEED 300 SQ. METERS WITH A MINIMUM OF 24 HOURS BETWEEN ADJACENT AREAS OF PLACEMENT CONSTRUCTION JOINTS SHALL NOT BE FARTHER APART THAN 8.00 METERS IN ANY DIRECTION, ALL SLABS ON GRADE SHALL BE SAW CUT ON EACH GRID LINE AND MID BAY LINE (IN BOTH DIRECTIONS) WITHIN 24 HOURS AFTER CASTING.
- E. PROVIDE SAW-CUT & CONSTRUCTION JOINTS.





CONTROL JOINTS CAN BE EITHER CONSTRUCTION JOINTS OR WEAKENED PLANE JOINTS. FIGURE 4

F. NOTES ON FOUNDATION

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ARCH. HANNAH FAITH P. MORTA, uap

MERIAN P. CATAJAY-MANI Ed. D., CESE

Director IPPDO

REVIEWED BY:

APPROVED BY:

A SAFE BEARING CAPACITY (SEP) LISED IN THE DESIGN = 30 KGA AT A MINIMUM DETH OF 2.00 FROM THE NOTIFICATION OF A DESIGN OF THE STREET AND A DESIGN OF A DESIGN OF

- B. FOUNDATIONS ARE DESIGNED BASED ON GEOTECHNICAL EVALUATION REPORT BY THE GEOTECHNICAL ENGINEER. CONTRACTOR SHALL REPORT TO ENGINEER ON RECORD THE ACTUAL SOIL CAPACITY PRIOR TO CONSTRUCTIONS
- C. STRUCTURAL COLUMN WILL REST ON SPREAD FOOTING REFER TO FOOTING SCHEDULE
- E. PROVIDE TEMPORARY REMOVAL OF WATER FROM ANY SOURCE DURING CONSTRUCTION
- F. CONTRACTOR SHALL DESIGN, INSTALL AND MONITOR EXCAVATIONS RETENTION SYSTEMS, AS REQUIRED FOR PROTECTION OF ADJACENT PROPERTIES AND PROVIDE ALL MEASURES AND PRECAUTIONS NECESSARY TO MINIMUZE SETTLEMENT AND PREVENT DANAGE TO ADJACENT EXISTING ON EWE CONSTRUCTION.

H. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL ELEVATOR DETAILS, REFER TO ARCHITECTURAL, MCCHANICAL, PLUMBING AND OTHER TRADES FOR SUBSOIL DRAWINGS SYSTEM, MACHINERY ANCHORS AND OTHER EMBEDDED TIEMS, DEPRESSIONS, FINISHES, DOWELS FOR MASONRY WALLS, CURBS, ETC.

J. SEE TYPICAL DETAIL OF LIMITING SLOPE OF ADJACENT FOOTING AT DIFFERENT ELEVATION (REFER TO FIGURE 5)

DETAIL OF LIMITING SLOPE

OF ADJACENT FOOTINGS

FIGURE 5

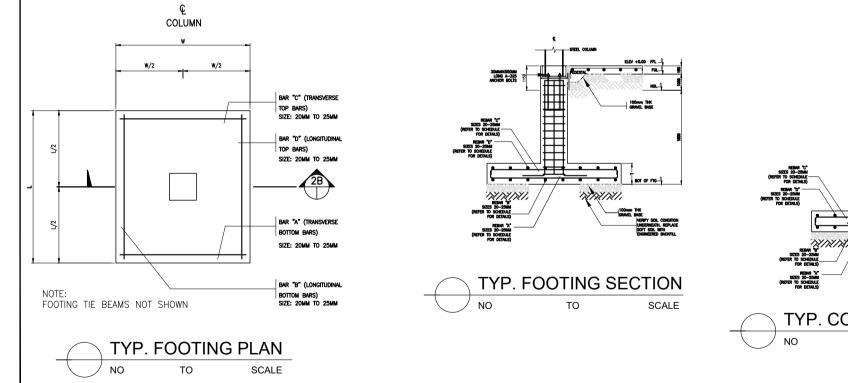
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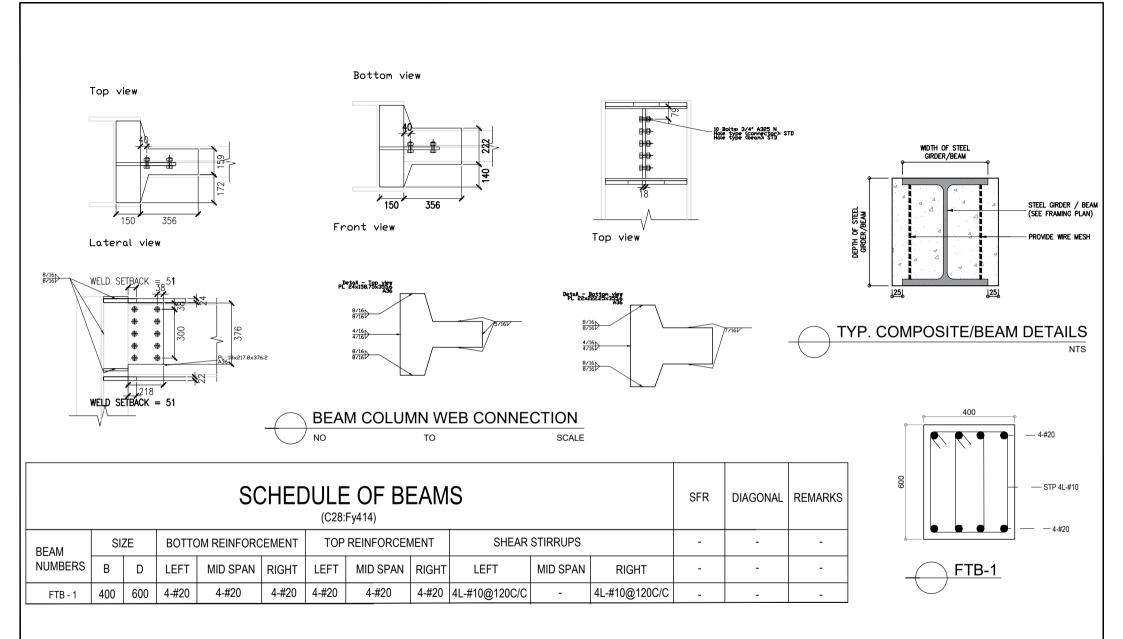
G. PREPARE CONDITIONS OF CONCRETE SUPPLY AND PLACEMENT OF THE COMPLETE FOUNDATION FOR THE FULL THICKNESS AS A CONTINUOUS MONOLITHIC CASTING. H. DO NOT BACKFILL AGAINST BASEMENT WALLS UNTIL GROUND FLOOR SLABS HAVE BEEN PLACED AND THE CONCRETE HAS ATTAINED THE REQUIRED STRENGTH.

SCHEDULE OF FOOTINGS											
FOOTING	FOOTING DIMENSIONS (mm)					REINFC	RCEMENT (
DESIG.	D DEPTH	L LENGTH	W WIDTH	T THICKNESS	BAR "A"	BAR "B"	BAR "C"	BAR "D"	REMARKS		
F1	2500	2600	2600	500	20ø@230	20ø@230	20ø@230	20ø@230	ISOLATED		
CF1	2500	4500	2400	500	20ø@230	20ø@230	20ø@230	20ø@230	COMBINED FOOTING		
CF2	2500	6150	2400	500	20ø@245	20ø@245	20ø@245	20ø@245	COMBINED FOOTING		
CF3	2500	21200	4000	500	20ø@125	20ø@125	20ø@245	20ø@245	COMBINED FOOTING		

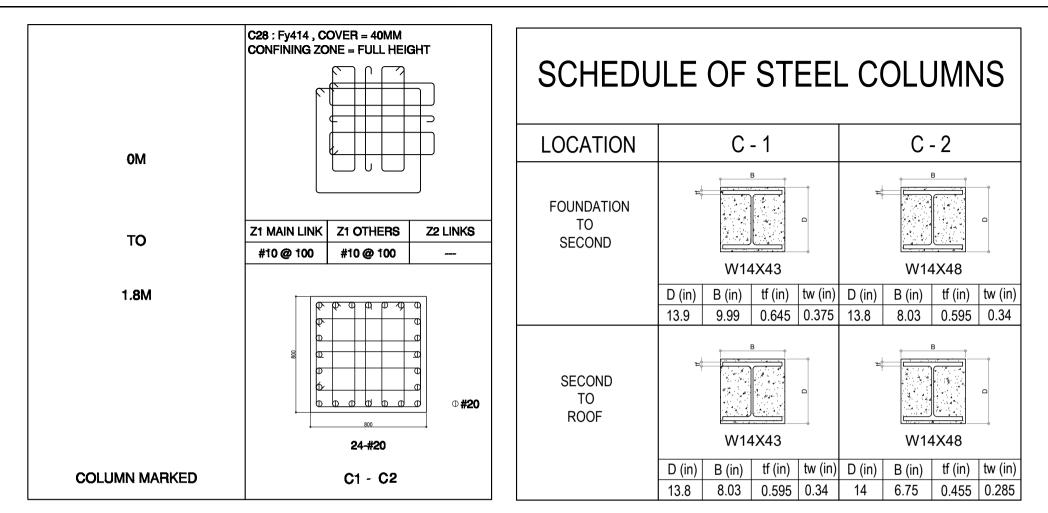


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	COMBINED FC	OTING SECTION
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COLUMN AND WALL SCHEDULE

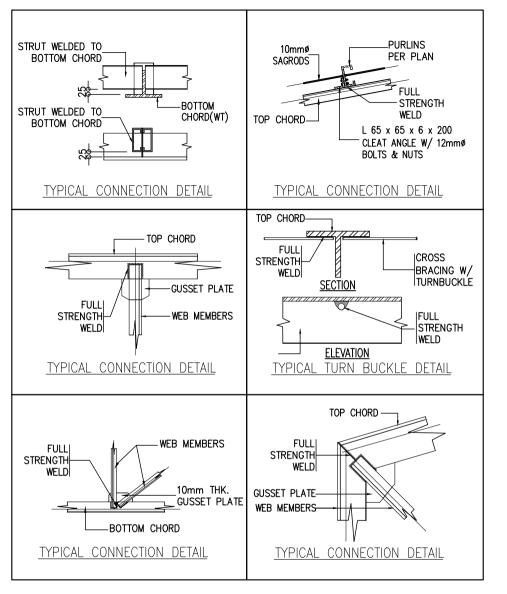
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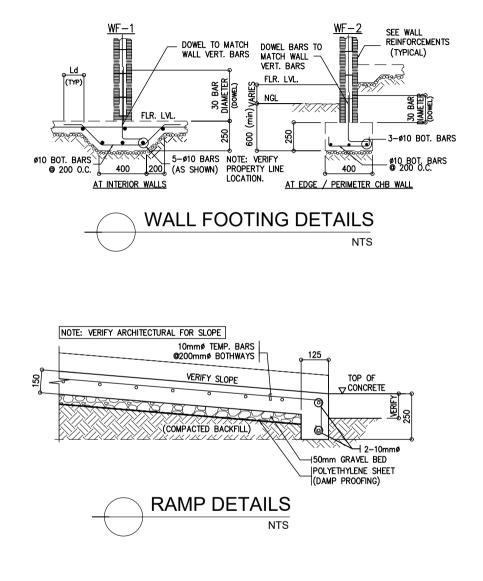
NOTES:

1. BE = BOUNDARY ELEMENT AS PER ACI 318. PROVIDE CONFINING REINFORCEMENT ACROSS ENTIRE HEIGHT OF WALL IN THE BOUNDARY ELEMENT

2. Z1 = SPECIAL CONFINING ZONE AS PER ACI 318, Z2 = REMAINING ZONES AS PER ACI 318

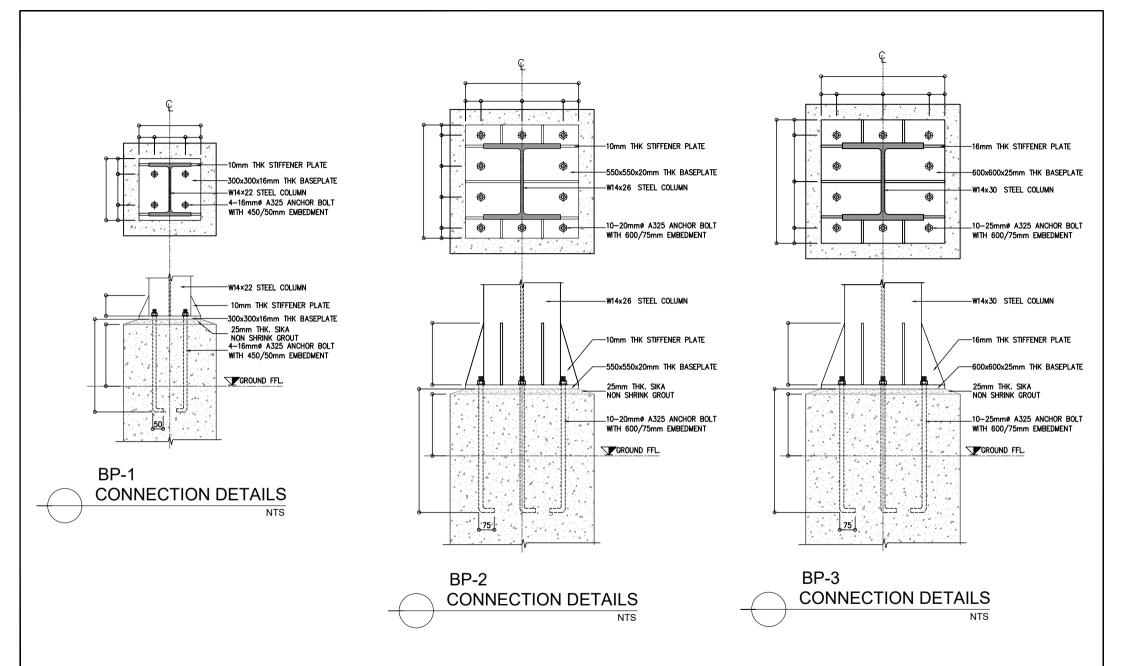
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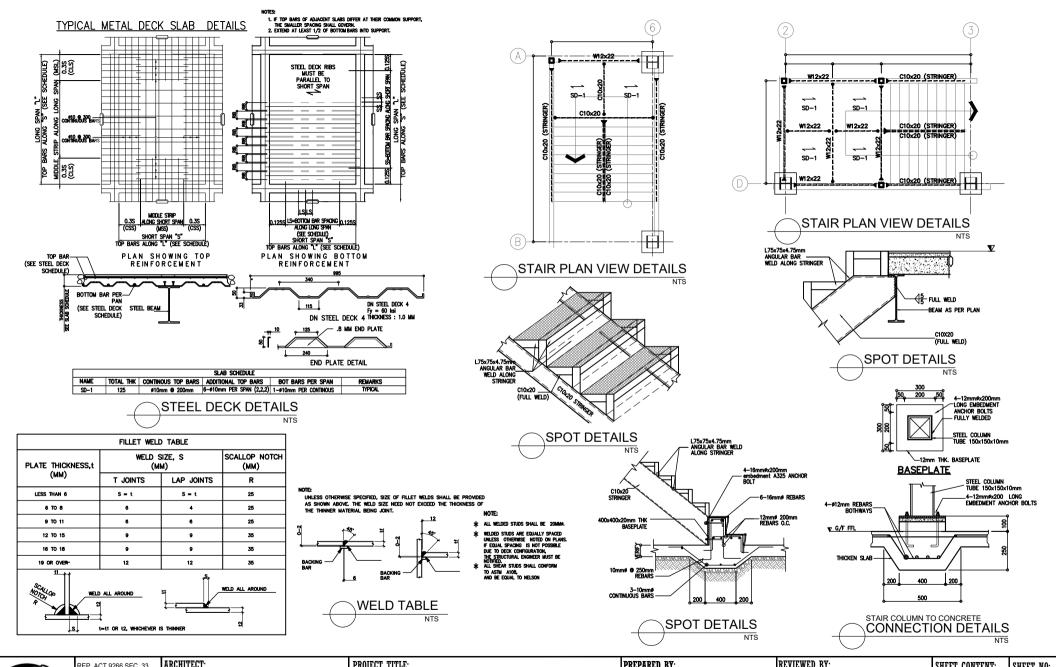




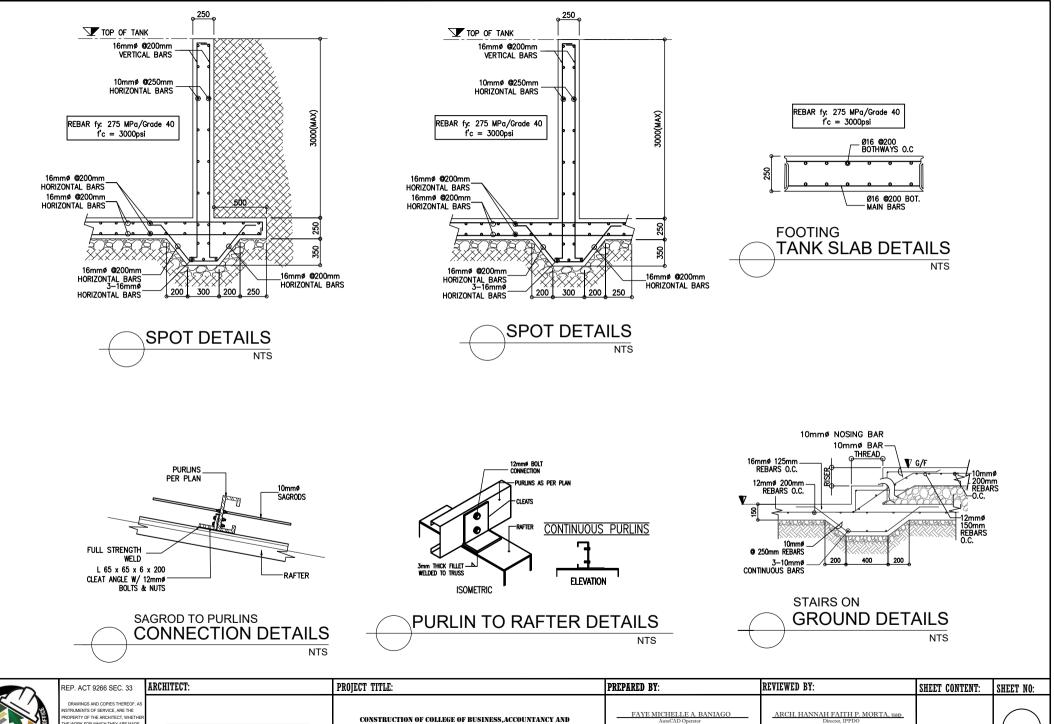
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